

Prime Consulting Engineers Pty. Ltd.

Design Report:

Heavy Duty (Square Umbrella) for 90km/hr Wind Speed

For



Ref: R-25-1346-1

Date: 29/04/2025

Amendment: -

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1 Introduction and Scope:

The report and certification are the sole property of Prime Consulting Engineers Pty. Ltd.

Prime Consulting Engineers have been engaged by Ultra Shade to carry out a structural analysis of **4.8m square** Umbrella Structures for **90km/hr** in open condition as a worse case scenario. It should be noted that the outcome of our analysis is limited to the selected items as outlined in this report.

This report shall be read in conjunction with the documents listed in the references (Cl. 1.2)

1.1 Project Description

The report examines the effect of the peak gust wind that an equivalent moving average time of approximately 0.2S for **25 m/s (90 km/hr)** positioned for the worst effect, in open and closed conditions respectively, on **4.8m square** Umbrella Structures as the worst-case scenario. The relevant Australian Standards AS1170.0:2002 General principles, AS1170.1:2002 Permanent, imposed, and other actions and AS1170.2:2021 Wind actions are used. The design check is in accordance with AS1664.1 Aluminium Structures.

1.2 References

- The documents referred to in this report are as follows:
 - o Report on results produced through SAP2000 V24 software & excel spreadsheets.
- The basic standards used in this report are as follows:
 - AS 1170.0:2002 Structural Design Actions (Part 0: General principles)
 - AS 1170.1:2002 Structural Design Actions (Part 1: Permanent, imposed, and other actions)
 - AS 1170.2:2021 Structural Design Actions (Part 2: Wind Actions)
 - AS1664.1:1997 Aluminium Structures.
- Section Properties of Aluminium Section provided by the client.
- The program(s) used for this analysis are as follows:
 - o SAP2000 V24
 - Microsoft Excel

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1.3 Notation

AS/NZS Australian Standard/New Zealand Standard

FEM/FEA Finite Element Method/Finite Element Analysis

SLS Serviceability Limit State

ULS Ultimate Limit State

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2 Design Overview

2.1 Geometry Data



Square Heavy Duty Umbrella Dimensions

Size	A	В	С	D	Е	F	G	Н
2.5 m	2350	2850	2500	3550	1250	3600	1750	400
2.8 m	2300	2850	2830	4050	1415	3600	1550	400
2.9 m	2250	2850	2960	4200	1480	3600	1450	400
3.0 m	2250	2900	3060	4350	1530	3600	1350	400
3.6m	2300	2950	3670	5100	1835	3700	1050	450
4.0m	2300	3100	4000	5600	2000	4000	1100	450
4.5 m	2350	3250	4400	6200	2200	4150	900	450
4.8 m	2300	3300	4700	6600	2350	4150	700	450

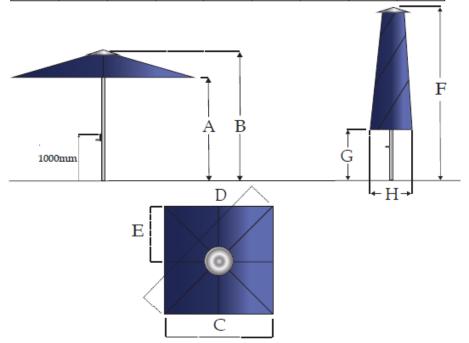


Figure 1: Geometry of the square Umbrella



2.2 Assumptions & Limitations

- For forecast winds in excess of **80km/hr**, the umbrella structure should be closed.
- The structure is design for wind parameters as below:
 - Wind Region A
 - o TC2
 - \circ M_s, M_t & M_d = 1
- Shall the site conditions/wind parameters exceed prescribed design wind actions (refer to <u>Cl.4</u>), Prime Consulting Engineers Pty. Ltd. should be informed to determine appropriate wind classifications and amend computations accordingly.
- It is assumed that the fabric weighs 830gr/m².
- Aluminium alloy is to be **Alloy 6106-T6** for **post, and Alloy 6061 T6** for **arms** and **braces**.
- The **Post** is specified as **82.5** x **5.2** mm, as outlined in the Heavy-Duty Umbrella Specifications.
- The **Arm** size is specified as **25 x 50 x 3.2 mm**, as per the Heavy-Duty Umbrella Specifications.
- The **Brace** size is specified as **25 x 50 x 3.2 mm**, as per the Heavy-Duty Umbrella Specifications.
- It is assumed that the umbrella is "empty under" for calculating wind loads. As per AS1170.2:2021, empty under is defined "Any goods or materials stored under the roof block less than 50% of the cross-section exposed to the wind".

2.3 Exclusions

- Design of fabric.
- Wind actions due to tropical or severe tropical cyclonic areas.
- Snow and ice loads.
- Footing design.

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2.4 Design Parameters and Inputs

2.4.1 Load Cases

1. G Permanent actions (Dead load)

2. Wu Ultimate wind action (ULS)

3. Ws Serviceability wind action (SLS)

2.4.2 Load Combinations

Strength (ULS):

1. 1.35G Permanent action only

2. 0.9G+W_u Permanent and wind actions

3. 1.2G+W_u Permanent and wind actions

Serviceability (SLS):

1. G+W_s Wind service actions

3 Specifications

3.1 Material Properties

				M	aterial	Propertie	S			
6106-T6	F _{tu}	F _{ty}	F _{cy}	Fsu	F _{sy}	F _{bu}	F _{by}	E	k _t	k _o
0100-10	235	210	210	144	120	480	337	70000	1	1.12

					Mate	rial Prope	rties			
6061-T6	F _{tu}	F _{ty}	F _{cy}	F _{su}	F _{sy}	F _{bu}	F _{by}	Е	k _t	k _c
0001-10	262	241	241	165	138	551	386	70000	1	1.12

3.2 Buckling Constants

TAB BUCKLING CONSTA	LE 3.3(D) NTS FOR	ALLOY 610)6-T6			
Type of member and stress	Interce	ept, MPa	Slop	e, MPa	Inter	section
Compression in columns and beam flanges	Вс	234.44	D _c	1.36	Cc	70.85
Compression in flat plates	Bp	267.52	Dp	1.65	Cp	66.32
Compression in round tubes under axial end load	Bt	257.80	Dt	8.85	Ct	*
Compressive bending stress in rectangular bars	B _{br}	395.01	D _{br}	3.63	C _{br}	72.46
Compressive bending stress in round tubes	B _{tb}	517.50	D _{tb}	37.34	C _{tb}	83.09
Shear stress in flat plates	Bs	153.44	Ds	0.72	Cs	87.57
Ultimate strength of flat plates in compression	k ₁	0.35	k ₂	2.27		
Ultimate strength of flat plates in bending	k ₁	0.5	k ₂	2.04		

 $^{^*}$ C_t shall be determined using a plot of curves of limit state stress based on elastic and inelastic buckling or by trial-and-error solution

TAB BUCKLING CONSTA	LE 3.3(D) NTS FOR		61-T6			
Type of member and stress	Interc	ept, MPa	Slop	e, MPa	Inter	section
Compression in columns and beam flanges	Вс	271.04	Dc	1.69	Cc	65.89

Compression in flat plates	B _p	310.11	Dp	2.06	Cp	61.60
Compression in round tubes under axial end load	B _t	297.39	Dt	10.70	Ct	*
Compressive bending stress in rectangular bars	B _{br}	459.89	D _{br}	4.57	C _{br}	67.16
Compressive bending stress in round tubes	B _{tb}	653.34	D _{tb}	50.95	C _{tb}	78.23
Shear stress in flat plates	B _s	178.29	Ds	0.90	Cs	81.24
Ultimate strength of flat plates in compression	K 1	0.35	k ₂	2.27		
Ultimate strength of flat plates in bending	<i>k</i> ₁	0.5	k ₂	2.04		

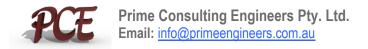
 $^{^*}$ C_t shall be determined using a plot of curves of limit state stress based on elastic and inelastic buckling or by trial-and-error solution

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3.3 Member Sizes & Section Properties

MEMBER(S)	Section	d	t	Уc	Ag	Z _x	Z _y	S _x	Sy	I _x	ly	J	r _x	гу
		mm	mm	mm	mm²	mm³	mm³	mm³	mm³	mm ⁴	mm ⁴	mm ⁴	mm	mm
Post 82.5 x 5.2	D 82.5 x5.2	82.5	5.2	41.3	1262.8	22968.8	22968.8	31118.4	31118.4	947463.7	947464	1894927.5	27.4	27.4

MEMBER(S)	Section	b	d	t	Уc	Ag	Z _x	Z _y	S _x	Sy	I _x	l _y	J	Γ _x	гу
		mm	mm	mm	mm	mm²	mm³	mm³	mm³	mm³	mm ⁴	mm ⁴	mm⁴	mm	mm
Arms	25x50x3.2	25	50	3.2	25.0	439.0	5278.0	3337.9	6785.5	4041.5	131949.8	41724.2	97109.2	17.3	9.7
Brace	25x50x3.2	25	50	3.2	25.0	439.0	5278.0	3337.9	6785.5	4041.5	131949.8	41724.2	97109.2	17.3	9.7



4 Wind Analysis

4.1 Wind calculations

Project: Ultra shade-heavy duty

PCE

Job no. 25-1346-1

Designer: JK

PRIME CONSULTING ENGINEERS PTY. LTD

Date: 30/04/2025

Amendment:

Name	Symbol	Value	Unit	Notes	Ref.
		Inj	out		
Importance level		2			Table 3.1 - Table 3.2 (AS1170.0)
Annual probability of exceedance		Temporary			Table 3.3
Regional gust wind speed		90	Km/hr		
Regional gust wind speed	V_{R}	25	m/s		
Wind Direction Multipliers	M_{d}	1			Table 3.2 (AS1170.2)
Terrain Category	TC	2			
Terrain Category Multiplier	Mz,cat	0.91			
Shield Multiplier	Ms	1			4.3 (AS1170.2)
Topographic Multiplier	M_{t}	1			4.4 (AS1170.2)
Site Wind Speed	$V_{\text{Site},\beta}$	22.75	m/s	$V_{Site,\beta}=V_R*M_d*M_{z,cat}*M_S,M_t$	
Pitch	α	22.5	Deg		
Pitch	α	-	rad		
Width	В	4.8	m		
Length	D	4.8	m		
Height	Z	2.65	m		
Porosity Ratio	δ	1		ratio of solid area to total area	
		Wind P	ressure		
hoair	ρ	1.2	Kg/m ³		



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dynamic response factor	C_dyn	1			
Nind Pressure	ho*Cfig	0.311	Kg/m ²	ρ =0.5 ρ air*($V_{des,\beta}$) ² * C_{fig} * C_{dyn}	2.4 (AS1170.2)
		WIND DIRE	ECTION 1 (6)=0)	
		Externa	al Pressure		
1. Free Roof				α =0°	
Area Reduction Factor	Ka	1			D7
local pressure factor	K _I	1			
porous cladding reduction factor	K _p	1.00			
External Pressure Coefficient MIN	C _{P,w}	-0.3			
External Pressure Coefficient MAX	C _{P,w}	0.6			
External Pressure Coefficient MIN	C _{P,I}	-0.6			
External Pressure Coefficient MAX	C _{P,I}	0			
		-0.30			
aerodynamic shape factor MIN	C _{fig,w}				
aerodynamic shape factor MAX	$C_{\text{fig,w}}$	0.60			
aerodynamic shape factor MIN	$C_{\text{fig,I}}$	-0.60			
aerodynamic shape factor MAX	$C_{\text{fig,I}}$	0.00			
Pressure Windward MIN	Р	-0.09	kPa		
Pressure Windward MAX	Р	0.19	kPa		
Pressure Leeward MIN	Р	-0.19	kPa		
Pressure Leeward MAX	Р	0.00	kPa		
		WIND DIRE	CTION 2 (0		
		WIND DIRE	d Pressure	•	
		LAterna	ii i ressure	•	
4. Free Roof				α =180 °	D7
Area Reduction Factor	Ka	1			
local pressure factor	K_{l}	1			
porous cladding reduction factor	K_p	1.00			
External Pressure Coefficient MIN	$C_{P,w}$	-0.3			
External Pressure Coefficient MAX	$C_{P,w}$	0.4			
External Pressure Coefficient MIN	$C_{P,I}$	-0.4			
External Pressure Coefficient MAX	$C_{P,I}$	0			
aerodynamic shape factor MIN	$C_{\text{fig,w}}$	-0.30			
aerodynamic shape factor MAX	$C_{\text{fig,w}}$	0.40			
aerodynamic shape factor MIN	$C_{\text{fig,I}}$	-0.40			
aerodynamic shape factor MAX	$C_{fig,I}$	0.00			

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Pressure MIN (Windward Side)	Р	-0.09	kPa
Pressure MAX (Windward Side)	Р	0.12	kPa
Pressure MIN (Leeward Side)	Р	-0.12	kPa
Pressure MAX (Leeward Side)	Р	0.00	kPa
,	•	-	

4.1.1 Summary

WIND EXTERNAL	Dire	ction1	Direction2		
PRESSURE	Min (Kpa)	Max (Kpa)	Min (Kpa)	Max (Kpa)	
Windward	-0.093	0.186	-0.093	0.124	
Leeward	-0.186	0.000	-0.124	0.000	

4.2 Wind Load Diagrams

4.2.1 Wind Load Ultimate (W_{min})

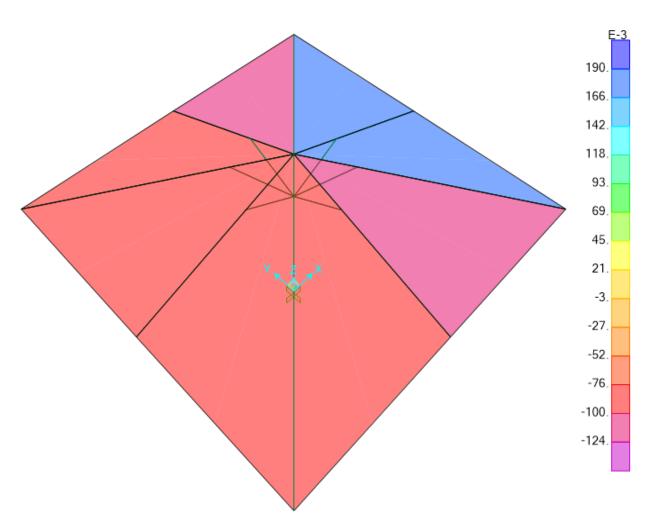
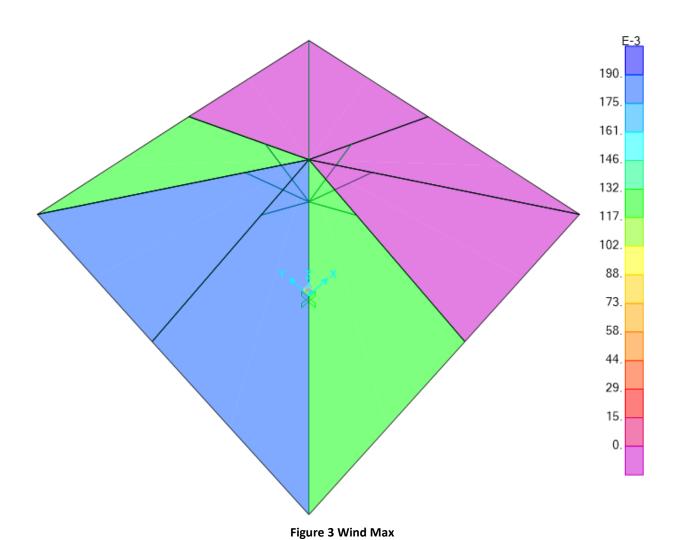


Figure 2 Wind Min (KN, m)

4.2.2 Wind Load Ultimate (W_{max})



5 Analysis

5.1 Results

5.1.1 Maximum Bending Moment in Major Axis

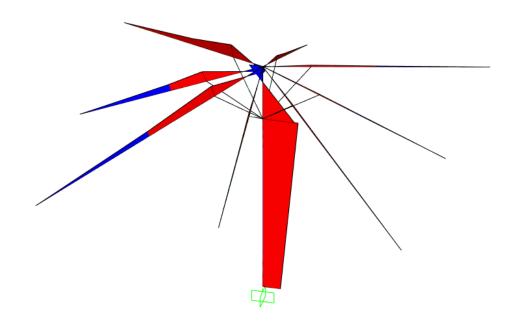


Figure 4 Maximum Bending Moment - Major

5.1.2 Maximum Bending Moment in Minor Axis

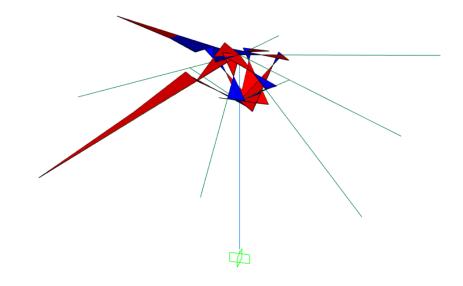


Figure 5: Maximum Bending Moment - Minor

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5.1.3 Maximum Shear

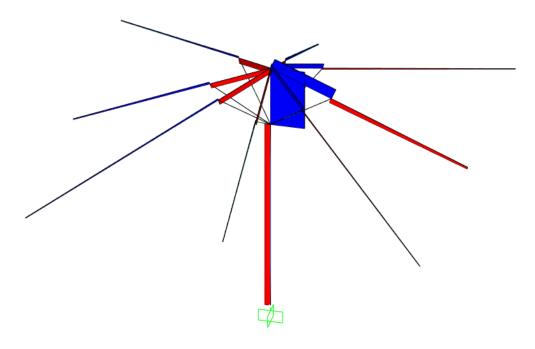


Figure 6 Maximum Shear

5.1.4 Maximum Axial Force

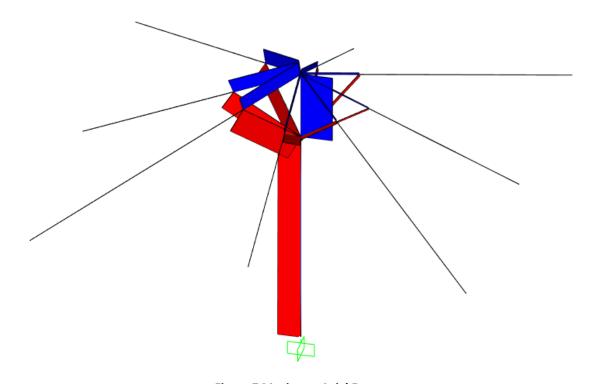


Figure 7 Maximum Axial Force

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5.1.5 Maximum Reactions

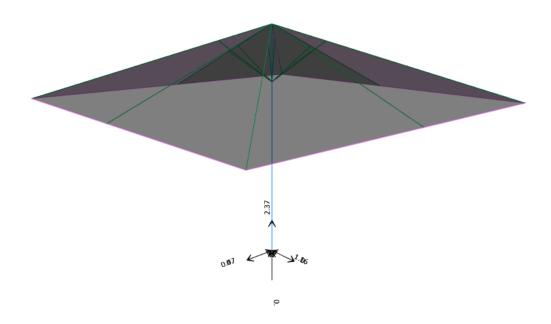


Figure 8 Maximum Reactions (opened)

 $\begin{aligned} &\text{Max F}_x = 0.7 \text{ kN} \\ &\text{Max F}_y = 0.7 \text{ kN} \\ &\text{Max F}_z = 2.56 \text{ kN} \\ &\text{Max M}_x = 1.16 \text{ kN.m} \\ &\text{Max M}_y = 1.16 \text{ kN.m} \end{aligned}$

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6 Aluminium Member Design

All Aluminium members passed. The summary results are tabulated below. Refer to <u>Appendix 'A'</u> for details.

MEMBER(S)	Section	d	t	V _x	V _y	P (Axial) Compression (-) Tension (+)	Mx	Му
		mm	mm	kN	kN	kN	kN.m	kN.m
Post 82.5 x 5.2	D 82.5 x5.2	82.5	5.2	-0.70	0.00	0.17	2.51	0.00

MEMBER(S)		b	d	t	V _x	V _y	P (Axial)	Mx	Му
		mm	mm	mm	kN	kN	kN	kN.m	kN.m
Arms	25x50x3.2	25	50	3.2	-0.48	0.03	0.02	-0.83	0.04
Brace	25x50x3.2	25	50	3.2	0.00	0.07	-0.85	0.00	0.10

7 Summary and Recommendations

- The 4.8m square umbrella structure as specified has been analyzed with a conclusion that it has the capacity to withstand wind speeds up to and including 90km/hr.
- For forecast winds in excess of **80km/hr** the structure should be completely folded.
- For uplift due to 90km/hr, 300kg (3 kN) holding down weight or 4/M12 HST3 M12 hef1, hnom = 60mm Mechanical Anchor or alternatively HIT-HY 200-R + HIT-V-F (8.8)
 M12, heff = 70mm chemical anchors to concrete slab supplied by HILTI Australia is required.
- The bearing pressure of soil should be clarified prior to any construction.

Yours faithfully, Prime Consulting Engineers Pty. Ltd. Bijaya Giri, MEng, MIEAust, CPEng, NER, APEC, IntPE (Aus), PE Vic

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8 Appendix A – Aluminium Design Based on AS1664.1

8.1 Post 82.5 x 5.2 mm



Job no. 25-1346-1 **Date**: 30/04/2022

NAME	SYMBOL		VALUE	UNIT	NOTES	REF
D 82.5 x5.2	Post 82.5 x 5.2					
Alloy and temper	6105-T5					AS1664. 1
Tension	F_tu	=	235	MPa	Ultimate	T3.3(A)
1 01101011	F_{ty}	=	210	MPa	Yield	
Compression	F _{cy}	=	210	MPa		
Shear	F _{su}	=	144	MPa	Ultimate	
Official	F_{sy}	=	120	MPa	Yield	
Bearing	F_bu	=	480	MPa	Ultimate	
Deaning	F_{by}	=	337	MPa	Yield	
Modulus of elasticity	Е	=	70000	MPa	Compressiv e	
	kt	=	1.0			T2 4/D)
	k c	=	1.1			T3.4(B)
FEM ANALYSIS RESULTS						
Axial force	Р	=	2.509902 2	kN	compressio n	
	Р	=	0	kN	Tension	
In plane moment	M_{x}	=	2.316680 6	kNm		
Out of plane moment	M_{y}	=	4.223E-14	kNm		
DESIGN STRESSES						
Gross cross section area	A_g	=	1262.794 6	mm²		
In-plane elastic section modulus	Z _x	=	22968.81	mm ³		
Out-of-plane elastic section mod.	Z_{y}	=	22968.81 8	mm³		
Stress from axial force	f _a	=	P/A _g			

Stress from in-plane bending	f_{bx}	= = =	1.99 0.00 M _x /Z _x	MPa MPa	compressio n Tension	
Strong from out of plane	f _{by}	=	100.86 M _y /Z _y	MPa	compressio n	
Stress from out-of-plane bending		=	0.00	MPa	compressio n	
Tension						
3.4.3 Tension in rectangular tub	es					3.4.3
	фF∟	= O R	233.42	MPa		
	φF∟	=	247.69	MPa		
COMPRESSION						
3.4.8 Compression in columns,1. General	axial, gross se	ection				3.4.8.1
Unsupported length of member	L	=	3240	mm		
Effective length factor	k	=	1.00			
Radius of gyration about buckling axis (Y)	r _y	=	27.39	mm		
Radius of gyration about buckling axis (X)	r _x	=	27.39	mm		
Slenderness ratio	kLb/ry	=	118.29			
Slenderness ratio	kL/rx	=	118.29			
Slenderness parameter	λ	=	2.062			
, , , , , , , , , , , , , , , , , , , ,	D _c *	=	77.8			
	S ₁ *	=	0.60			
	S_2^*	=	1.24			
	фсс	=	0.869			
Factored limit state stress	фГ∟	=	42.90	MPa		
2. Sections not subject to torsion	nal or torsiona	al-flexur	al buckling			3.4.8.2
Largest slenderness ratio for flexural buckling	kL/r	=	118.29			
3.4.11 Uniform compression in o	components o	f colum	ns, gross se	ection - flat		
Uniform compression in comportantes with both edges, walls of			ss section -	curved		3.4.11

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	k 1	=	0.35			T3.3(D)
mid-thickness radius of round tubular column or maximum	R_{m}	=	38.65			
mid-thickness radius						
	t	=	5.2	mm		
Slenderness	R _m /t	=	7.432692 3			
Limit 1	S ₁	=	0.62			
Limit 2	S_2	=	672.46			
Factored limit state stress	φFL	=	210.68	MPa		
Most adverse compressive limit state stress	Fa	=	42.90	MPa		
Most adverse tensile limit state stress	Fa	=	233.42	MPa		
Most adverse compressive & Tensile capacity factor	f _a /F _a	=	0.05		PASS	
BENDING - IN-PLANE						
3.4.13 Compression in beams, ex	ktreme fibre,	gross s	section round	l or oval		
tubes	·	Ü				
Unbraced length for bending	L_b	=	3240	mm		
Second moment of area (weak axis)	ly	=	9.47E+05	mm ⁴		
Torsion modulus	J	=	1.89E+06	mm³		
Elastic section modulus	Z	=	22968.81 8	${\rm mm^3}$		
	R _b /t	=	7.43			
Limit 1	S ₁	=	42.32			
Limit 2	S_2	=	83.09			
Factored limit state stress	φFL	=	233.42	MPa		3.4.13
3.4.18 Compression in componer edges supported	nts of beams	s - curv	erd plates wi	th both		
	\mathbf{k}_1	=	0.5			T3.3(D)
	k_2	=	2.04			T3.3(D)
mid-thickness radius of round tubular column or maximum mid-thickness radius	R_b	=	38.65	mm		
ma unomisso radias	t	=	5.2	mm		
Slenderness	R _b /t	=	7.432692			
I	-		3		I	

Limit 1	S ₁	=	3.61			
Limit 2	S_2	=	83.09			
Factored limit state stress	φFL	=	198.63	MPa		
Most adverse in-plane bending limit state stress	F _{bx}	=	198.63	MPa		
Most adverse in-plane bending capacity factor	f_{bx}/F_{bx}	=	0.51		PASS	
BENDING - OUT-OF-PLANE NOTE: Limit state stresses, ϕF_L (doubly symmetric section)	are the same	for out-	-of-plane be	nding		
Factored limit state stress	фҒ∟	=	198.63	MPa		
Most adverse out-of-plane bending limit state stress	F _{by}	=	198.63	MPa	_	
Most adverse out-of-plane bending capacity factor	f_{by}/F_{by}	=	0.00		PASS	
COMBINED ACTIONS 4.1.1 Combined compression an	nd bending					4.1.1
	Fa	=	42.90	MPa		3.4.11
	Fao	=	210.68	MPa		3.4.11
	F_bx	=	198.63	MPa		3.4.18
	F_by	=	198.63	MPa		3.4.18
	f _a /F _a	=	0.046			
Check:	$f_a/F_a + f_{bx}/F_{bx}$	x + f _{by} /F _b				4.1.1
i.e.	0.55	≤	1.0		PASS	
SHEAR						
3.4.24 Shear in webs (Major Axis)						3.4.24
	R	=	41.25	mm		
	t	=	5.2	mm		
Equivalent h/t	h/t	=	31.50			
Limit 1	S_1	=	29.82			
Limit 2	S ₂	=	59.31			
Factored limit state stress	φF_{L}	=	112.64	MPa		

Stress From Shear force	f _{sx}	=	V/A _w 0.74	MPa		
3.4.25 Shear in webs (Minor Axis)						3.4.24
Clear web height	R	=	41.25	mm		
Equivalent h/t	t h/t	=	5.2 31.50	mm		
Factored limit state stress	фҒ∟	=	112.64	MPa		
Stress From Shear force	\mathbf{f}_{sy}	=	V/A_{w}			
			0.00	MPa		
Most adverse shear capacity factor (Major Axis)	f _{sx} /F _{sx}	=	0.01	МРа		
Most adverse shear capacity factor (Minor Axis)	f _{sy} /F _{sy}	=	0.00	Мра	PASS	
COMBINED ACTIONS						
4.4 Combined Shear, Compression	on and bend	ding				4.4
Check:	f _a /F _a + f _b /F _b -	+ (f _s /F _{s)} ²	≤ 1.0			
i.e.	0.55	≤	1.0		PASS	

8.2 Arms (25 x 50 x 3.2 mm)



Job no. 25-1346-1 **Date**: 30/04/2025

NAME	SYMBOL		VALUE	UNIT	NOTES	REF
25x50x3.2	Arms					
Alloy and temper	6061-T6					AS1664.1
Tension	F_{tu}	=	262	MPa	Ultimate	T3.3(A)
1 61131011	F_{ty}	=	241	MPa	Yield	
Compression	F_{cy}	=	241	MPa		
Chaor	F_{su}	=	165	MPa	Ultimate	
Shear	F_{sy}	=	138	MPa	Yield	
Bearing	F_bu	=	551	MPa	Ultimate	
Dearing	F_{by}	=	386	MPa	Yield	

Modulus of elasticity	E	=	70000	MPa	Compressive	
Modulus of elasticity	_	_	70000	IVIFA	Compressive	
	\mathbf{k}_{t}	=	1			T3.4(B)
	k c	=	1			10.1(5)
FEM ANALYSIS RESULTS						
Axial force	Р	=	0	kN	compression	
	Р	=	0.0167837	kN	Tension	
In plane moment	M_{x}	=	0.8290739	kNm		
Out of plane moment	M_{y}	=	0.0441878	kNm		
DESIGN STRESSES						
Gross cross section area	A_g	=	439.04	mm²		
In-plane elastic section modulus	Z_{x}	=	5277.9916	mm³		
Out-of-plane elastic section mod.	Z_{y}	=	3337.9352	${\rm mm^3}$		
Stress from axial force	fa	=	P/A _g			
		=	0.00	MPa	compression	
Otropo forms in allowable and in a		=	0.04	MPa	Tension	
Stress from in-plane bending	f _{bx}	=	M _x /Z _x 157.08	MPa	compression	
Stress from out-of-plane	$\mathbf{f}_{\mathbf{b}\mathbf{y}}$	=	M_y/Z_y	IVIFA	compression	
bending	ю	=	13.24	MPa	compression	
Tension					,	
3.4.3 Tension in rectangular tube	s					
	φF _L	= OR	228.95	MPa		
	φF _L	=	222.70	MPa		
COMPRESSION						
3.4.8 Compression in columns, a	xial, gross	sectio	n			
1. General						3.4.8.1
Unsupported length of member	L	=	3550	mm		
Effective length factor	k	=	1.00			
<u> </u>					I	
Radius of gyration about buckling axis (Y)	r _y	=	9.75	mm		
Radius of gyration about	r _y r _x	=	9.75 17.34	mm		
Radius of gyration about buckling axis (Y) Radius of gyration about	•					

1					İ	ı
Slenderness parameter	λ	=	5.355			
	D_c^*	=	90.3			
	S_1^*	=	0.33			
	S_2^*	=	1.23			
	фсс	=	0.950			
Factored limit state stress	фГ∟	=	7.98	MPa		
2. Sections not subject to torsional	al or torsio	nal-fle	xural bucklind	ק		3.4.8.2
Largest slenderness ratio for			_	,		
flexural buckling	kL/r	=	286.71			
3.4.10 Uniform compression in co	mponents	s of col	lumns, gross	section -		
flat plates			-			
1. Uniform compression in compo	nents of c	column	s, gross secti	ion - flat		3.4.10.1
plates with both edges supported	k 1	_	0.35			T3.3(D)
May distance between tops of	K 1	=	0.33			13.3(D)
Max. distance between toes of fillets of supporting elements	b'	=	18.6			
for plate	J	_	10.0			
·	t	=	3.2	mm		
Slenderness	b/t	=	5.8125			
Limit 1	S_1	=	12.34			
Limit 2	S_2	=	32.87			
Factored limit state stress	φFL	=	228.95	MPa		
Most adverse compressive limit			7.00	MDe		
state stress	Fa	=	7.98	MPa		
Most adverse tensile limit state stress	Fa	=	222.70	MPa		
Most adverse compressive & Tensile capacity factor	f _a /F _a	=	0.00		PASS	
BENDING - IN-PLANE						
3.4.15 Compression in beams, ex tubes, box sections	treme fibr	e, gros	ss section rec	tangular		
Unbraced length for bending	L_b	=	2795	mm		
Second moment of area (weak axis)	ly	=	4.17E+04	mm ⁴		
Torsion modulus	J	=	9.71E+04	mm³		
Elastic section modulus	Z	=	5277.9916	mm ³		
Slenderness	S	=	463.51	111111		
Limit 1	S ₁	_	0.39			
Little 1	J 1	=	0.38		l	1



					i	
Limit 2	S ₂	=	1695.86			
Factored limit state stress	φFL	=	181.00	MPa		3.4.15(2)
3.4.17 Compression in componer						
compression), gross section - flat	•			orted		T0.0(D)
	k ₁	=	0.5			T3.3(D)
	k_2	=	2.04			T3.3(D)
Max. distance between toes of fillets of supporting elements for plate	b'	=	18.6	mm		
·	t	=	3.2	mm		
Slenderness	b/t	=	5.8125			
Limit 1	S_1	=	12.34			
Limit 2	S_2	=	46.95			
Factored limit state stress	φFL	=	228.95	MPa		
Most adverse in-plane bending	F _{bx}		181.00	MPa		
limit state stress	Гbх	=	101.00	IVIFA		
Most adverse in-plane bending capacity factor	f_{bx}/F_{bx}	=	0.87		PASS	
BENDING - OUT-OF-PLANE						
NOTE: Limit state stresses, φF _L a (doubly symmetric section)	are the san	ne for c	out-of-plane	bending		
Factored limit state stress	φF _L	=	181.00	MPa		
Most adverse out-of-plane bending limit state stress	F _{by}	=	181.00	MPa		
Most adverse out-of-plane bending capacity factor	f _{by} /F _{by}	=	0.07		PASS	
COMBINED ACTIONS						4.4.4(0)
4.1.1 Combined compression and	a bending					4.1.1(2)
	Fa	=	7.98	MPa		3.4.8
	F _{ao}		228.95	MPa		3.4.10
	F _{ao}					3.4.10
		=	181.00	MPa		
	F_{by}	=	181.00	MPa		3.4.17
	f _a /F _a	=	0.000			
Check:	$f_a/F_a + f_{bx}/$	F _{bx} + f _b	$_{\text{by}}/F_{\text{by}} \leq 1.0$			(3)

i.	.e. 0.94	. ≤	1.0		PASS	
SHEAR						
3.4.24 Shear in webs (Major Axis)						4.1.1(2)
Clear web height	h	=	43.6	mm		
-	t	=	3.2	mm		
Slenderness	h/t	=	13.625			
Limit 1	S ₁	=	29.01			
Limit 2	S ₂	=	59.31			
Factored limit state stress	φFι	. =	131.10	MPa		
Stress From Shear force	f _{sx}	=	V/A _w			
			1.32	MPa		
3.4.25 Shear in webs (Minor Axis)						
Clear web height	b	=	18.6	mm		
-	t	=	3.2	mm		
Slenderness	b/t	=	5.8125			
Factored limit state stress	φГι	_ =	131.10	MPa		
Stress From Shear force	f_{sy}	=	V/A_w			
			0.07	MPa		
Most adverse shear capacity factor (Major Axis)	f _{sx} /F _s	sx =	0.01	MPa		
Most adverse shear capacity factor (Minor Axis)	f _{sy} /F _s	sy =	0.00	Мра	PASS	
COMBINED ACTIONS						
4.4 Combined Shear, Compre	ession and	bendina				
and a comment of the compre		cag				
Chec	ck: f _a /F _a +	f _b /F _b + (f	$s/F_{s})^2 \le 1.0$			
i.	.e. 0.87	′ ≤	1.0		PASS	

8.3 Brace (25 x 50 x 3.2 mm)



Job no. 25-1346-1 **Date**: 30/04/2025

NAME	SYMBOL		VALUE	UNIT	NOTES	REF
25x50x3.2	Brace					
Alloy and temper	6061-T6					AS1664.1
Tension	Ftu	=	262	MPa	Ultimate	T3.3(A)
Tonoion	F_{ty}	=	241	MPa	Yield	
Compression	F_{cy}	=	241	MPa		
Shear	F_su	=	165	MPa	Ultimate	
Official	F_{sy}	=	138	MPa	Yield	
Bearing	F_bu	=	551	MPa	Ultimate	
Dearing	F_{by}	=	386	MPa	Yield	
Modulus of elasticity	E	=	70000	MPa	Compressiv e	
	k t	=	1			
	k _c	=	1			T3.4(B)
FEM ANALYSIS RESULTS						
Axial force	Р	=	0.853674 3	kN	compression	
	Р	=	0	kN	Tension	
In plane moment	M_{x}	=	0	kNm		
Out of plane moment	M_{y}	=	0.103245 1	kNm		
DESIGN STRESSES						
Gross cross section area	A_g	=	439.04	mm²		
In-plane elastic section modulus	Z_{x}	=	5277.991 6	mm³		
Out-of-plane elastic section mod.	\mathbf{Z}_{y}	=	3337.935 2	mm³		
Stress from axial force	fa	=	P/A _g			
		=	1.94	MPa	compression	
Otropo francia migra haratira		=	0.00	MPa	Tension	
Stress from in-plane bending	f _{bx}	=	M _x /Z _x 0.00	MPa	compression	
		=	0.00	IVIFA	compression	l

Stress from out-of-plane	f _{by}	=	M_y/Z_y			
bending	-,	=	30.93	MPa	compression	
Tension						
3.4.3 Tension in rectangular tube	S					
	фҒ∟	=	228.95	MPa		
		O R				
	φF _L	=	222.70	MPa		
COMPRESSION						
3.4.8 Compression in columns, a	xial, gross s	section				
1. General						3.4.8.1
Unsupported length of member	L	=	1000	mm		
Effective length factor	k	=	1.00			
Radius of gyration about buckling axis (Y)	r y	=	9.75	mm		
Radius of gyration about buckling axis (X)	r _x	=	17.34	mm		
Slenderness ratio	kLb/ry	=	102.58			
Slenderness ratio	kL/rx	=	57.68			
Slenderness parameter	λ	=	1.92			
·	D_c^*	=	90.3			
	S_1^*	=	0.33			
	S_2^*	=	1.23			
	фсс	=	0.848			
Factored limit state stress	φF _L	=	55.69	MPa		
2. Sections not subject to torsion	al or torsion	al-flexu	ıral buckling			3.4.8.2
Largest slenderness ratio for	kL/r	=	102.58			
flexural buckling	KL/I	_	102.00			
3.4.10 Uniform compression in co	omponents (of colur	nns, gross s	section -		
Uniform compression in compo plates with both edges supported		olumns,	gross section	on - flat		 3.4.10.1
, mar mar a sugar supported	k 1	=	0.35			T3.3(D)
Max. distance between toes of						
fillets of supporting elements for plate	b'	=	18.6			
	t	=	3.2	mm		
Slenderness	b/t	=	5.8125			
Limit 1	S_1	=	12.34			

Limit 2	S ₂	=	32.87			
Factored limit state stress	фҒ∟	=	228.95	MPa		
Most adverse compressive limit state stress	Fa	=	55.69	MPa		
Most adverse tensile limit state stress	Fa	=	222.70	MPa		
Most adverse compressive & Tensile capacity factor	f _a /F _a	=	0.03		PASS	
BENDING - IN-PLANE 3.4.15 Compression in beams, extubes, box sections	treme fibre	, gross	s section rect	angular		
Unbraced length for bending	L _b	=	1000	mm		
Second moment of area (weak axis)	ly	=	4.17E+04	mm ⁴		
Torsion modulus	J	=	9.71E+04	mm³		
Elastic section modulus	Z	=	5277.991 6	mm³		
Slenderness	S	=	165.83			
Limit 1	S_1	=	0.39			
Limit 2	S ₂	=	1695.86			
Factored limit state stress	φF _L	=	200.85	MPa		3.4.15(2)
3.4.17 Compression in component compression), gross section - flat						
	k ₁	=	0.5			T3.3(D)
	k ₂	=	2.04			T3.3(D)
Max. distance between toes of	_					()
fillets of supporting elements for plate	b'	=	18.6	mm		
	t	=	3.2	mm		
Slenderness	b/t	=	5.8125			
Limit 1	S ₁	=	12.34			
Limit 2	S_2	=	46.95			
Factored limit state stress	φF _L	=	228.95	MPa		
Most adverse in-plane bending limit state stress	F _{bx}	=	200.85	MPa		
Most adverse in-plane bending capacity factor	f _{bx} /F _{bx}	=	0.00		PASS	

NOTE: Limit state stresses, φF _L (doubly symmetric section)	are the sam	e for ou	ıt-of-plane b	ending		
Factored limit state stress	фҒ∟	=	200.85	MPa		
Most adverse out-of-plane bending limit state stress	F _{by}	=	200.85	MPa		
Most adverse out-of-plane bending capacity factor	f_{by}/F_{by}	=	0.15		PASS	
COMBINED ACTIONS						
4.1.1 Combined compression a	nd bending					4.1.1(2
	Fa	=	55.69	MPa		3.4.8
	Fao	=	228.95	MPa		3.4.10
	F _{bx}	=	200.85	MPa		3.4.17
	F_by	=	200.85	MPa		3.4.17
	f _a /F _a	=	0.035			
Check:	$f_a/F_a + f_{bx}/F$	$F_{\rm bx} + f_{\rm by}/$	F _{by} ≤ 1.0			4.1.
i.e.	0.19	≤	1.0		PASS	(3
SHEAR						
3.4.24 Shear in webs (Major Axis)						4.1.1(2
Clear web height	h	=	43.6	mm		
	t	=	3.2	mm		
Slenderness	h/t	=	13.625			
Limit 1	S_1	=	29.01			
Limit 2	S ₂	=	59.31			
Factored limit state stress	φF∟	=	131.10	MPa		
Stress From Shear force	f _{sx}	=	V/A _w	MDo		
3.4.25 Shear in webs (Minor Axis)			0.01	MPa		
Clear web height	b	=	18.6	mm		
	t	=	3.2	mm	1	

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Factored limit state stress Stress From Shear force	фF _L f _{sy}	=	131.10 V/A _w 0.19	MPa MPa	
Most adverse shear capacity factor (Major Axis)	f _{sx} /F _{sx}	=	0.00	MPa	
Most adverse shear capacity factor (Minor Axis)	f_{sy}/F_{sy}	=	0.00	Мра	PASS
COMBINED ACTIONS					
4.4 Combined Shear, Compress	ion and bend	ding			
Check:	$f_a/F_a + f_b/F_b$	+ (f _s /F _s	$(a)^2 \le 1.0$		
i.e.	0.19	≤	1.0		PASS

Phone: (02) 8964 1818

9 Appendix B – Technical Data Sheet



A.B.N. 77 010 472 563 56 Zillmere Road, Boondall QLD 4034 P.O. Box 856, Virginia QLD 4014 Tel: (07) 3265 7288

Em: info@ultrashade.com.au

Heavy Duty Umbrella Specifications

Frame Specifications:

Post Specifications: 82.5mm x 5.2mm Round Tube 6106 T6 Aluminium

Arm Specification: 25mm x 50mm x 3.2mm 6106 T5 Aluminium

Winding Mechanism: Anti-tamper Rack and Pinion Gearbox

Materials: Aluminum Frame Components, Nylon Arm ends and Bushes, Stainless Steel Fittings

Finish: Dulux Powder Coat Range

Standard Frame Colours: White, Beige, Primrose, Green, Black Charcoal, Silver, White Birch,

Claret, Navy **Custom Colours Available on Request**

Available Base Types: Stainless Steel Bolt down or In-ground (Heavy Duty Umbrellas must be

fixed to the ground)

Fabric Types:

100% Solution Dyed Acrylic Outdoor Awning Fabric

Large Colour Selection
UV Blockout UV Reflective

Water Resistant

Mould & Mildew Resistant Fade Resistant Guarantee

Serge Ferrari Soltis Proof 502V2 Satin

Only available up to 5.0m Octagonal and 4.0m Square

Satin finish

Reinforced dirt resistance and easy clean-ability

Heat and weather protection

High UV resistance

High dimensional stability due to Precontraint® technology





UltraShade

A.B.N. 77 010 472 563

56 Zillmere Rd, Boondall QLD 4034 P.O. Box 856, Virginia QLD 4014

Tel: (07) 3265 7288 Fax: (07) 3265 7304

Em: info@ultrashade.com.au



Heavy Duty Umbrella

UltraShade Price List

Effective 1st July 2023

OCTAGONAL	Size	Coverage	Price Excluding GST	Price Including GST
CE 3.0m	3.0m Diameter	6.3 m ²	\$4,616.36	\$5,078.00
CE 3.5m	3.5m Diameter	8.7 m^2	\$4,657.27	\$5,123.00
CE 4.0m	4.0m Diameter	11.3 m ²	\$4,970.91	\$5,468.00
CE 4.5m	4.5m Diameter	14.4 m ²	\$5,291.82	\$5,821.00
CE 5.0m	5.0m Diameter	17.6 m ²	\$5,635.45	\$6,199.00
CE 5.5m	5.5m Diameter	21.4 m ²	\$6,055.45	\$6,661.00
CE 6.0m	6.0m Diameter	25.5 m ²	\$6,404.64	\$7,044.00
<u>SQUARE</u>	Size	Coverage	Price Excluding GST	Price Including GST
CE 3.0m	3.0m x 3.0m	$9.0 \; m^2$	\$4,825.45	\$5,308.00
CE 3.6m	3.6m x 3.6m	12.9 m ²	\$5,140.00	\$5,654.00
CE 4.0m	4.0m x 4.0m	16.0 m ²	\$5,455.45	\$6,001.00
CE 4.5m	4.5m x 4.5m	20.2 m ²	\$5,770.00	\$6,347.00
CE 4.8m	4.8m x 4.8m	23.0 m ²	\$6,085.45	\$6,694.00

All umbrellas include an Acrylic Fabric Canopy with No Valance.

All above prices includes the choice of either a;

Stainless Steel Bolt Down Base or Stainless Steel In-ground Base

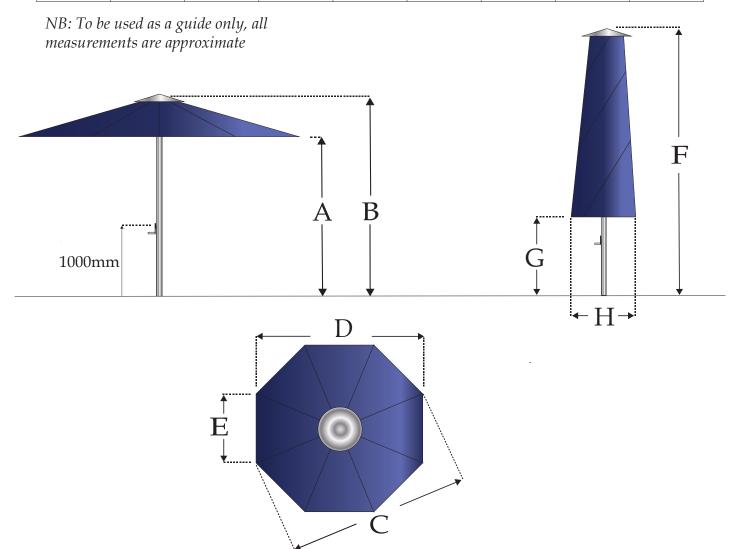
Optional Extras

Valance (Straight or Scalloped)	\$357.27	\$393.00
Serge Ferrari 502V2 Satin Surcharge	\$283.64	\$312.00
Extra Spigot Only	\$215.45	\$237.00
Extra Bolt Down Base Only	\$215.45	\$237.00
Extra In Ground Base Only	\$215.45	\$237.00
Velcro 4 sides - 8 x panels (Square only)	\$330.91	\$364.00
Velcro per panel (Square only)	\$47.27	\$52.00
Gutter	\$330.91	\$364.00
Acrylic Dust Cover with Zip Replacement	\$610.91	\$672.00
Handle	\$70.00	\$77.00
Installation	P.O.A.	P.O.A.



Octagonal Heavy Duty Umbrella Dimensions

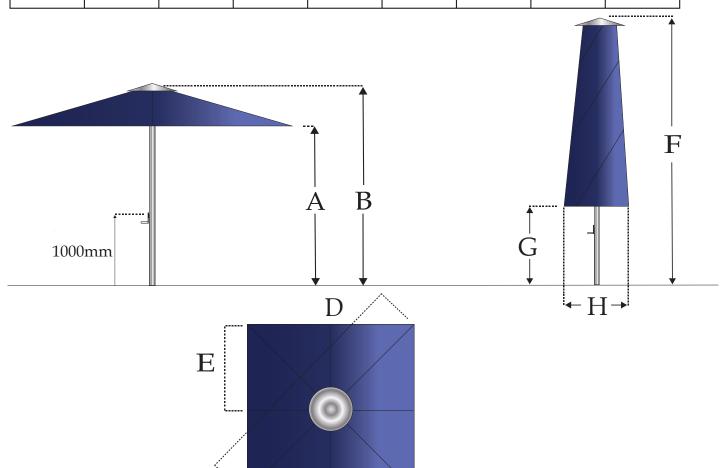
Size	A	В	С	D	Е	F	G	Н
3.5 m	2300	2850	3500	3200	1350	3650	1800	400
4.0 m	2300	2950	4000	3700	1500	3750	1650	400
4.5 m	2300	3000	4500	4150	1750	3900	1500	450
5.0 m	2400	3250	5000	4550	1900	4050	1450	450
5.5 m	2350	3200	5500	5100	2100	4100	1200	500
6.0 m	2400	3250	6000	5460	2300	4150	1050	500



UltraShade 77

Square Heavy Duty Umbrella Dimensions

Size	A	В	С	D	E	F	G	Н
2.5 m	2350	2850	2500	3550	1250	3600	1750	400
2.8 m	2300	2850	2830	4050	1415	3600	1550	400
2.9 m	2250	2850	2960	4200	1480	3600	1450	400
3.0 m	2250	2900	3060	4350	1530	3600	1350	400
3.6m	2300	2950	3670	5100	1835	3700	1050	450
4.0m	2300	3100	4000	5600	2000	4000	1100	450
4.5 m	2350	3250	4400	6200	2200	4150	900	450
4.8 m	2300	3300	4700	6600	2350	4150	700	450





P.O. Box 856, Virginia QLD 4014 Tel: (07) 3265 7288 Fax: (07) 3265 7304

Em: info@ultrashade.com.au

Heavy Duty Umbrella Installation Instructions

Bolt Down Installations - Fixing to Concrete:

Supplied Parts:

- a) The Base Plate (250mm x 250 mm x 12mm)
- b) Umbrella Spigot
- c) Five 3/8" x 1" Bolts
- d) Five 3/8" Spring Washers
- e) Two ½" Grub Screws
- f) One 1/4 "Allen Key
- g) One M8 x 90 mm Stainless Bolt& Nylock Nut
- h) Umbrella Hat
- i) Umbrella Handle

Recommended Fixings:

M12 x 100mm Dyna Bolts - Stainless Steel

M12 Chem Set - Stainless Steel

M12 Screw Bolt - Galvanized

Minimum Concrete Thickness:

Reinforced concrete slab 100mm thick.

PPE Required:

Dust mask, Safety Glasses, Hearing Protection, Gloves

Tools Required:

- 1. M12 Concrete SDS Drill
 Bit
- 2. SDS Drill
- 3. Post Level
- 4. Dust Extraction
- 5. Spanner/Socket Set
- 6. Cordless Drill
- 7. Tape Measure
- 8. 8mm Cobalt Drill Bit
- 9. 9/16" Spanner
- 10. 1 box M12 Flat Washer
- 1. Bolt the spigot to the base plate using the $3/8" \times 1"$ bolts and spring washers supplied using the 9/16" spanner.
- 2. Slide the spigot and the attached base up inside the umbrella post and lock in place using the Allen Key with the two supplied ½" Grub Screws
- 3. Attach the Hat to the umbrella using the threaded hole at the top of the umbrella with the 9/16" Spanner. Once the Hat is attached, do not lay umbrella down as to not damage the hat.
- 4. Stand the umbrella in approximate location using 3 people. Have one person on the base end and two people on the canopy end, carry the umbrella to the approximate position and rest the base on the ground while two people hold the umbrella up. Have someone hold the base so it does not slip whilst two people push the umbrella vertical. Open the umbrella by removing the strap, attaching the handle and pushing all 8 arms out. Tighten the umbrella and lock in place. Have a few people hold the umbrella and measure to final position.
- 5. Using the post level, ensure the umbrella is plumb. Use M12 Stainless Steel Flat Washers under the fixing holes to level the base plate if required. The fixing bolts will be able to pass through the washers and hold the packing in place.
- 6. Use the base plate as a template and drill through the base fixing holes to the required depth of your concrete fixings. Ensure to use correct dust extraction techniques.
- 7. Fix the umbrella down using Dyna bolts or similar and cap with dome nuts.
- 8. Loosen the ½" grub screws and rotate umbrella to desired orientation. Lock grub screws again with Allen Key.
- 9. Locate the 8mm hole in the umbrella post 50mm up from the bottom of the post.
- 10. Using the 8mm cobalt drill bit, drill through the spigot from one side of the umbrella, then drill through the other side of the umbrella then run the drill through both holes.
- 11. Secure the umbrella to the spigot with the M8 x 90 mm bolt and Nylock nut.
- 12. Check Spigot bolts are tight.
- 13. Tighten the umbrella and remove the handle. Enjoy your UltraShade Heavy Duty Umbrella!



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Heavy Duty Umbrella Installation Instructions

Bolt Down Installation - Fixing to Timber Deck:

Supplied Parts:

- a) The Base Plate
- b) Umbrella Spigot
- c) Five 3/8" x 1" Bolts
- d) Five 3/8" Spring Washers
- e) Two ½" Grub Screws
- f) One 1/4 "Allen Key
- g) One M8 x 90 mm Stainless Bolt & Nylock
- h) Umbrella Hat
- i) Umbrella Handle

Recommended Fixings:

M12 x 130mm Hex Head

Bolts - Stainless Steel

M12 x 150mm Head Bolts -

Stainless Steel

M12 Stainless Steel Nuts

PPE Required:

Dust mask, Safety Glasses, Hearing Protection, Gloves

Tools and Hardware Required:

- 1. M12 Long Series Drill Bit
- 2. M5 Long Series Drill Bit,
- 3. Cordless Drill
- 4. Impact Driver
- 5. Post Level
- 6. Dust Extraction
- 7. Spanner/Socket Set
- 8. Tape Measure
- 9. 8mm Cobalt Drill Bit
- 10. 9/16" Spanner
- 11. 1 box M12 S/S Flat Washer
- 12. 75mm or 100mm Batton Screws
- 13. Length of 75mm x 100mm hardwood.
- 1. Bolt the spigot to the base plate using the $3/8" \times 1"$ bolts and spring washers supplied using the 9/16" spanner.
- 2. Locate the final position of the umbrella. It is ideal to have the base fit between two joists rather than sitting over the top of a joist.
- 3. With the base in its final position and orientation, use the base a template and drill through your decking boards through the base fixing holes with a M12 drill. This marks where you will need to reinforce the deck.
- 4. Get under the deck and measure the distance between you joists and cut two lengths of 75×100 mm hardwood to those lengths.
- 5. Ensure the centre of the hardwood aligns with the holes drilled through the decking boards and pre-drill the joists and hardwood with a 5mm drill and fix to the joists using 75mm or 100mm batten screws.
- 6. From the top of the deck, drill through the hardwood reinforcing with the M12 drill bit.
- 7. Slide the spigot and the attached base up inside the umbrella post and lock in place using the Allen Key with the two supplied ½" Grub Screws.
- 8. Stand the umbrella in approximate location using 3 people. Have one person on the base end and two people on the canopy end, carry the umbrella to the approximate position and rest the base on the ground while two people hold the umbrella up. Have someone hold the base so it does not slip whilst two people push the umbrella vertical. Open the umbrella by removing the strap, attaching the handle and pushing all 8 arms out. Tighten the umbrella and lock open. Have a few people hold the umbrella and align the base to 12mm holes in the deck.
- 9. Using the post level, ensure the umbrella is plumb. Use M12 Stainless Steel Flat Washers under the fixing holes to level the base plate if required. The fixing bolts will be able to pass through the washers and hold the packing in place.
- 10. Insert the M12 Bolts into the base and through the holes in the deck and reinforcing hardwood.
- 11. From underneath the deck, fix the bolts down with a M12 washer and nut.
- 12. Loosen the ½" grub screws and rotate umbrella to desired orientation. Lock grub screws again with Allen Key.
- 13. Locate the 8mm hole in the umbrella post 50mm up from the bottom of the post.
- 14. Using the 8mm cobalt drill bit, drill through the spigot from one side of the umbrella, then drill through the other side of the umbrella then run the drill through both holes.
- 15. Secure the umbrella to the spigot with the M8 x 90 mm bolt and Nylock nut.
- 16. Check Spigot bolts are tight.
- 17. Tighten the umbrella and remove the handle. Enjoy your UltraShade Heavy Duty Umbrella!



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Heavy Duty Umbrella Installation Instructions

In-ground Installations - Paving, soil or loose-ground

Supplied Parts:

- a) The In-ground Base
- b) Umbrella Spigot
- c) Five 3/8" x 1" Bolts
- d) Five 3/8" Spring Washers
- e) Two 1/2" Grub Screws
- f) One ¼ "Allen Key
- g) One M8 x 90 mm Stainless Bolt& Nylock Nut
- h) Umbrella Hat
- i) Umbrella Handle

Recommended Fixings:

4 x 20 KG Bag Structural Concrete Premix (Rapid set is strongly discouraged)

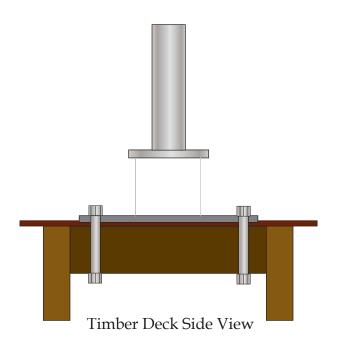
PPE Required:

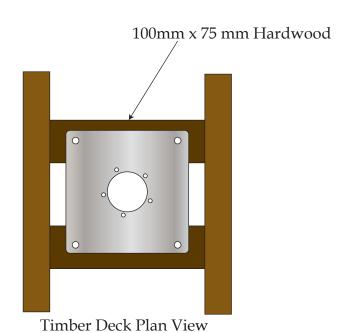
Dust mask, Safety Glasses, Hearing Protection, Gloves

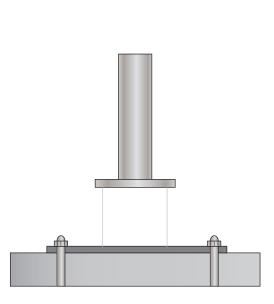
Tools and Hardware Required:

- 1. Post Hole Shovel
- 2. Spade
- 3. Crowbar or Jackhammer
- 4. Wheelbarrow
- 5. Cordless Drill
- 6. Post Level
- 7. Hose
- 8. Spanner/Socket Set
- 9. Tape Measure
- 10. 8mm Cobalt Drill Bit
- 11. 9/16" Spanner
- 1. Bolt the Spigot to the In-ground base using the $3/8" \times 1"$ Bolts and 3/8" Spring Washers and the 9/16" spanner.
- 2. Dig a hole for the concrete measuring approximately 400mm wide x 500mm deep. Note: The size of the hole depends on the soil type, (e.g., sandy soil = 500mm x 700mm). For taller umbrellas (custom height) = 500mm x 700mm.
- 3. Mix the required concrete and fill the hole until the level of the concrete is approximately 50mm below the ground level.
- 4. Insert the in-ground base into the concrete. The underside of the base flange should be **at least 12mm above**. ground level.
- 5. Using a post level, ensure the spigot is plumb while the concrete goes off.
- 6. UltraShade recommend leaving the concrete for 5 7 days to cure.
- 7. Remove the spigot from the in-ground base and insert into umbrella pole and lock in place with the two $\frac{1}{2}$ " grub screws and Allen key.
- 8. Attach the Hat to the umbrella using the threaded hole at the top of the umbrella with the 9/16" Spanner. Once the Hat is attached, do not lay umbrella down as to not damage the hat.
- 9. Stand the umbrella using 3 people. Have one person on the base/spigot end and two people on the canopy end, carry the umbrella to the in-ground base and rest the flange of the spigot on the in-ground base. Have someone hold the spigot down so it does not slip whilst two people push the umbrella vertical. Align the spigot holes with the base holes and secure the umbrella with the 3/8" x 1" Bolts and spring washers. Open the umbrella by removing the strap, attaching the handle and pushing all 8 arms out. Tighten the umbrella and lock open.
- 10. Loosen the $\frac{1}{2}$ " grub screws and rotate umbrella to desired orientation. Lock grub screws again with Allen Key.
- 11. Locate the 8mm hole in the umbrella post 50mm up from the bottom of the post.
- 12. Using the 8mm cobalt drill bit, drill through the spigot from one side of the umbrella, then drill through the other side of the umbrella then run the drill through both holes.
- 13. Secure the umbrella to the spigot with the M8 x 90 mm bolt and Nylock nut.
- 14. Check the spigot bolts are tight.
- 15. Tighten the umbrella and remove the handle. Enjoy your UltraShade Heavy Duty Umbrella!

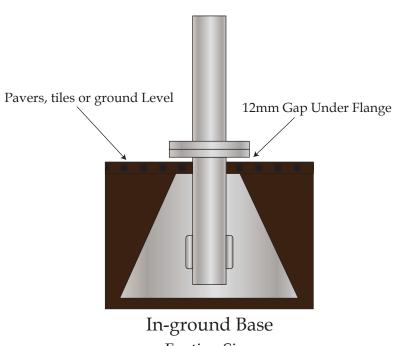
UltraShade







Bolt Down Base on Concrete or Tiles



Footing Size:
Approximately 400mm x 600 mm

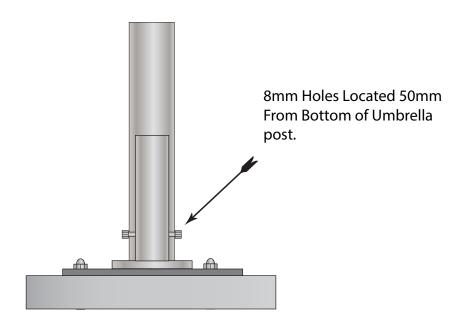


LOCKING BOLT INSTALLATION INFORMATION

To ensure the umbrella remains secure and does not turn or lift off the spigot, locate the 8mm holes in the umbrella post, 50mm from the bottom. Then use an 8mm drill bit to drill through the umbrella post then through the spigot. Drill through one side of the post, then drill through the opposite side, then pass the drill bit through both holes to ensure a bolt can pass through.

Once the holes have been drilled, secure the umbrella to the spigot by passing the supplied M8 x 90mm Bolt through the drilled holes and secure it with the provided M8 Nut using two 13mm Spanners.

NB: If this precaution is not taken when first installing the umbrella it could cause damage or injury due to turning or lifting off the spigot.





How to Open and Close Your UltraShade Heavy Duty Umbrella

To Open:

- 1. While lightly holding one arm out as far as possible, gently push all of the remaining 7 umbrella arms out from the main post.
- 2. The umbrella should unfold mostly by itself. If it fails to open, at least one of the arms will be in the over locked position.
- 3. Bolt the handle to the round stainless-steel axle and wind it up.
- 4. Open the umbrella until the canopy is taut.
- 5. Secure the umbrella by placing the locking pin into the round stainless-steel axle and into the post.
- 6. Store the handle in a safe place.
- 7. Ensure that the canopy is taut to prevent damage from stress fracturing due to flapping.

To Close:

- 1. Attach the handle to the round stainless-steel axle.
- 2. Release the pressure on the locking pin by using your body weight to turn the umbrella in a clock-wise direction.
- 3. Remove the locking pin from the axle and main post.
- 4. Wind the umbrella down with the handle in an anti-clockwise direction.
- 5. Ensure that all the canopy material is not jammed between the arms and arm brackets or between the arms and the bottom bracket system, and push the arms toward the main post. Failure to remove the fabric from the pinch points will result in damage to the canopy and will not be covered under warranty.
- 6. Wrap all the canopy material around the umbrella in the same way you would for a handheld rain umbrella.
- 7. Secure it with the provided strap.
- 8. Remove and store the handle in a safe place.

Note: It's important to secure the panels with the strap provided to prevent damage to the canopy, which is not covered by our warranty. If the umbrella is left in the down position for more than 24 hours, wind may cause flapping of the panels, which could result in fabric stress fracturing. This damage is not covered by warranty.

You can find an instructional video at: www.youtube.com/ultrashadeumbrellas



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Congratulations on your new umbrella! To prolong the life of your umbrella canopy, it is important to care for it properly. Regular cleaning and re treatment of the fabric will keep it looking great and extend its life. Here are some tips for caring for canvas and acrylic fabrics:

For new canvas installations, erect the umbrella fully and leave it erected for at least 2 to 4 days to allow the canvas to condition.

Ensure the canopy is thoroughly dry before closing it away to avoid encouraging algae, mildew, or other fungal growth.

Brush the fabric regularly with a soft brush and hose it down occasionally with clear, cold water to remove dust and grime. Do not let dirt, dust, grime, leaf litter, or bird matter remain on the fabric.

Cleaning Acrylic Fabrics:

Brush off any loose dirt and hose down the fabric.

Clean the fabric with a mild soap in lukewarm water (37°C). Rinse thoroughly and do not use detergents.

For more stubborn stains, soak the fabric for approximately 20 minutes in a solution of no more than 1/2 cup of non-chlorine bleach and 1/4 cup natural soap per 4 liters of water at approximately 30-35°C. Rinse thoroughly in cold water to remove all the soap.

Allow the fabric to air dry and apply an air-curing re-treatment to restore water repellent.

To aid in the care of your fabric awnings and blinds, we recommend using Brellaguard 303 Fabric Cleaner for acrylic fabrics. For restoring water repellent to acrylic fabric products, use 303 Fabric Guard Water Repellent for all woven fabrics.

To clean PVC fabrics, used in some outdoor umbrellas, follow these steps:

Remove any loose dirt or debris from the fabric surface using a soft brush or a dry cloth.

Prepare a cleaning solution by mixing warm water with a mild detergent. Use a non-abrasive and non-solvent cleaner.

Apply the cleaning solution to the fabric surface using a soft brush or sponge, working in a circular motion. Avoid using abrasive brushes or pads, as these can damage the fabric.

Rinse the fabric thoroughly with clean water, using a garden hose or a bucket of clean water.

Wipe off excess water from the fabric using a dry cloth, and allow the fabric to air dry completely before storing or using again.

For stubborn stains or marks, you may need to use a specialized cleaner recommended by the manufacturer. Be sure to follow the instructions carefully and test the cleaner on a small, inconspicuous area of the fabric first to ensure that it doesn't cause damage or discoloration.

If you have any questions about caring for your umbrella canopy, speak with one of our friendly customer service staff.



HEAVY DUTY UMBRELLA WARRANTY

Aluminum Frames are covered by a 5 Year structural warranty. Warranty specifically excludes damage to the powder coat finish caused by corrosion, scratching, pitting, fading or peeling.

Canopy Fabrics are covered by a 3 Year manufactures and workmanship warranty. Warranty specifically excludes; stains, mildew and fading.

Conditions of Warranty:

- 1. This warranty does not cover any repairs consequent upon accident, alterations or repairs by any other than an authorised dealer/agent of UltraShade, misuse, fire, floods, earthquakes or excessively high wind conditions.
- **2.** This warranty applies to the original purchaser from the purchase date and covers manufacturing faults and defects.
- **3.** This warranty is valid only for installation made by UltraShade/ Agent/ Dealer/Yourself/ Tradesman: The installation must be carried out exactly as shown in the Ultrashade Installation Instructions and Technical Information.
- **4.** Owner to ascertain position of all underground pipes and electrical wires and notify installer of any obstacles. Although all care will be taken, no responsibility can be accepted for any underground breakages.
- **5.** This warranty is only valid if the Locking pin is installed correctly and both pin and strap are used correctly at all times whilst the umbrella is erected and collapsed. Umbrella should not be closed for more than a 24 hour period.
- **6.** This warranty is valid only if the canopy has been kept clean and free of dust/debris with regular hosing as this will help prolong the life of the canopy.
- **7.** This warranty is only valid if the umbrella is put up and collapsed as detailed in the Technical information.
- **8.** The cost of transportation and insurance both ways for any repair to the UltraShade Umbrella is to be paid by the claimant.
- **9.** Warranty specifically excludes general wear and tear, rusting of steel components and parts, corrosion and damage caused as a result of failure to observe reasonable care, maintenance and assembly instructions.
- **10.** UltraShade reserves the right to determine whether or not fault is caused by faulty workmanship or material or any other part is defective.
- **11.** UltraShade may offer advice but accepts no responsibility to the suitability of the ultimate position of the UltraShade Umbrellas.

The benefits conferred by this manufacturer's warranty are in addition to all rights and remedies conveyed by the Competition and Consumer Act 2010 (Commonwealth), and any other statutory rights to which you may already be entitled, and this warranty does not exclude, restrict or modify any such rights or remedies that are implied by law.